St. Vincent Mercy Medical Center Heart Pavilion

Toledo, Ohio

Thesis Proposal:

Optimization of the Lateral System and Interior Member Sizes



Kristen Marie Lechner | Advisor: Dr. Linda Hanagan | Structural Option | December 12, 2008

TABLE OF CONTENTS

Table of Contents
Executive Summary
Introduction: St. Vincent Mercy Medical Center Heart Pavilion4
Existing Structural Description5
Floor System5
Roof System6
Columns6
Lateral System6
Foundation System8
Problem Statement9
Proposed Solution
Structural Depth: New Lateral System10
Breadth Study I: Façade Study10
Breadth Study II: Construction Management
Tasks & Tools
Timeline 14

EXECUTIVE SUMMARY

St. Vincent Mercy Medical Center Heart Pavilion is a four story hospital that provides diagnostics, surgery, and patient care. It was constructed for St. Vincent's Mercy Medical Center Campus, established in 1855, in downtown Toledo, Ohio.

The facility is approximately 144,000 square feet and reaches a height of 57'5" above grade with a typical floor to floor height of approximately 14 feet. A typical interior bay is 30 feet by 35 feet and is comprised of composite steel with a concrete slab on deck. Non-seismic steel moment frames are utilized to resist lateral forces at every column in both directions. This structural system was evaluated in previous technical reports and it was determined that the system meets architectural, strength, and serviceability requirements. Upon investigation of the soil classification within the site, it was determined that the soil was classified as Site Class E. As a result, it was concluded that the best solution for the Heart Pavilion is a structural steel system as this material is lighter than concrete.

The current site of St. Vincent Mercy Medical Center Heart Pavilion was chosen by the owner because it was already owned by Mercy Health Partners and it is adjacent to the main hospital. For these reasons, the Heart Pavilion will be kept on the existing site.

This thesis proposal outlines steps that will be taken to optimize the existing lateral system as well as interior member sizes. Classified as Site Class E, the soil is very sensitive to seismic forces. As a result, it was necessary at the time of design to place non-seismic steel moment frames at every column in both directions. To optimize this lateral system, seismically detailed steel moment frames will be utilized to resist lateral loads. Structural computer modeling will be used to complete this design. In addition, the seismically detailed connections will be designed based on applicable limit states. The use of geopiers will also be researched and designed. By utilizing this foundation system, lateral pressure within the soil will be increased, creating vertical reinforcement for the soil.

In the original design concepts, a surgery suit and conference spaces were located on the third floor. To provide natural day lighting to these spaces, a curtain wall is utilized along this entire level. However, the surgery spaces were later moved and patient rooms were added in its place. Therefore, the façade of the third floor will be changed to the brick veneer system. Heat loss calculations will be provided and conclusions will then be drawn on which system is more efficient for the Heart Pavilion's needs.

Implementing special detailing of the lateral system will require special welders to install the connections and special inspectors to regularly visit the construction site. For this reason, a construction schedule will be provided for the existing lateral system as well as the proposed lateral system. Conclusions will then be drawn based on viability of the new lateral system with respect to cost and constructability.

INTRODUCTION: ST. VINCENT MERCY MEDICAL CENTER HEART PAVILION

St. Vincent's Heart Pavilion is one of the seven hospitals that comprise Mercy Health Partners. As Toledo's first and only facility for the treatment of vascular disease, St. Vincent's Heart Pavilion has become a staple within the community. St. Vincent's Mercy Medical Center Campus is now able to take a leadership role in providing education to its students as well as saving lives through the treatment of vascular disease.

Modernization is emphasized through the façade of St. Vincent Mercy Medical Center Heart Pavilion. As one approaches the building from the North, a beautiful curtain wall composed of curved aluminum and spandrel glass is seen, thus adding great verticality to the building. As the eye gazes along the façade, stone bands and brick veneer promote horizontal progression to an attractive vertical component of stairs wrapped in stone veneer and spandrel glass. The eye is then led to the pedestrian bridge, connecting the Heart Pavilion to a parking garage, which shows off its structure through exposed chevron bracing.

The structure of the Heart Pavilion is comprised of a composite steel floor system that utilizes non-seismic steel moment frames to resist lateral forces. Drilled caissons and spread footings make up the foundation system. The ground floor is a reinforced slab on grade with grade beams between caissons to transfer wall load into the foundation.

The purpose of this thesis proposal is to communicate what has been learned from analyzing the existing conditions through three technical reports and propose an element to change. Optimization of the lateral system and interior member sizes will be the main theme of this proposal.



EXISTING STRUCTURAL DESCRIPTION

Floor System

St. Vincent Mercy Medical Center Heart Pavilion's typical floor system is made up of composite steel framing and normal weight concrete, creating a total floor thickness of $6\frac{1}{2}$ ". Composite action is created by the use of 2" 20 gauge steel deck with $5\frac{1}{2}$ " long, $3\frac{1}{4}$ " diameter shear studs evenly spaced over the length of each beam. Even though a composite system is used, the girders are actually non-composite. In order to avoid coping of the infill beams, the girders are placed 2" higher than the beams on a typical floor and $1\frac{1}{2}$ " higher on the roof as seen in Figure 1. This system saved money and fabrication time which resulted in faster steel erection. From analysis done in previous technical reports, it was found that strength requirements are met by approximately 4.2% while deflection criteria are met by approximately 26%. Please reference Figure 2 below to view a typical floor plan of the Heart Pavilion.

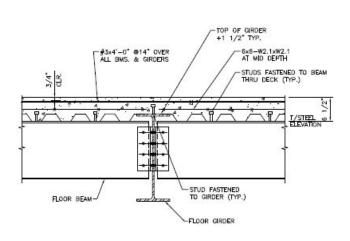


Figure 1: Detail of Existing Composite Steel Floor System

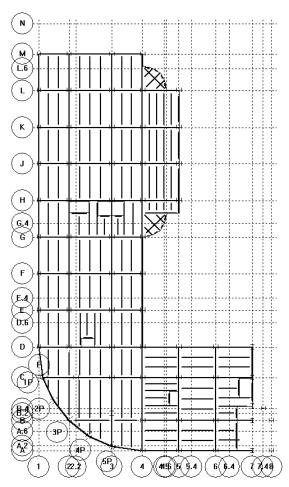


Figure 2: Typical Floor Layout

Roof System

The roof system is comprised W14x22 beams framing into W18X40 girders. This system was used as opposed to joists because surgery space was originally located directly below the main roof. The mechanical and medical equipment required for these spaces is easier to hang from wide flange beams.

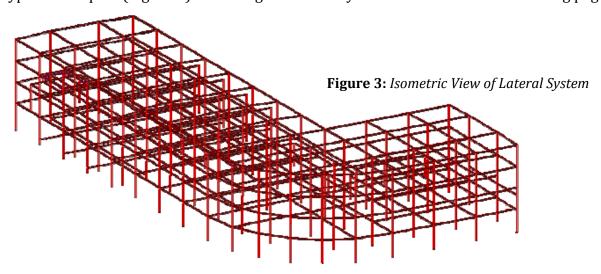
The roof envelope is surrounded by a parapet wall. The roof system is made up of two systems: $1\frac{1}{2}$ " x 22 GA. galvanized steel roof deck providing enclosure to the building, and 6 $\frac{1}{2}$ " normal weight concrete on $1\frac{1}{2}$ " 20 GA. galvanized metal deck providing support for the mechanical equipment located in the two penthouses. Both systems are topped with a single ply membrane, $\frac{3}{4}$ " plywood, and a minimum of 3" rigid insulation.

Columns

The columns used in St. Vincent Mercy Medical Center Heart Pavilion range from W10x119's to W12x210's, depending on their location within the building. While these sizes may seem large based purely on gravity, each column must resist induced moment since all columns are part of a moment frame. Pipe columns are used to support the roof for the main entrance lobby and the emergency vestibule canopy. All of the main building columns are spliced at the 2^{nd} - 3^{rd} floor. Base plates range in thickness from 1" to $2\frac{1}{4}$ " depending on which columns they are supporting. Each base plate utilizes a standard 4 bolt connection using either $\frac{3}{4}$ " A325 or $1\frac{1}{4}$ " A325 bolts.

Lateral System

At the time of design, braced frames were thought to be architecturally incompatible with this floor plan. As a result, steel moment frames were used for the lateral load resisting system. Please reference Figure 3 below to view an isometric view of the lateral system. A typical floor plan (Figure 4) indicating the lateral system is located on the following page.



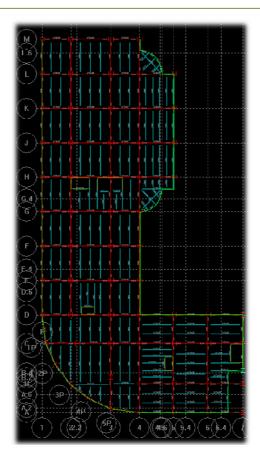


Figure 4: Typical Floor Plan Indicating Lateral System

The moment frames are connected in two different fashions as seen in Figures 5 and 6 below. The beam to column web moment connection is comprised of flange plates that are fillet welded to the column web and flange. The beam flanges are full-penetration welded to these plates. The beam to column flange moment connection utilizes double angles connecting the beam to the column flange, where the column flange is then full penetration welded to the beam flange.

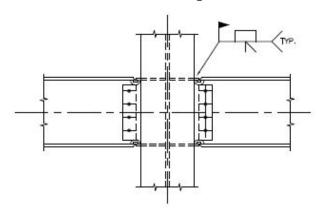


Figure 5: Beam to Column Web Connection

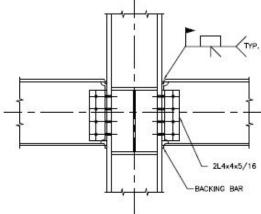


Figure 6: Beam to Column Flange Connection

Foundation System

The foundation system is made up of 80 drilled caissons and 6 spread footings that support the entrance lobby. The caisson caps are a uniform size of 4'x4'x3' thick. Between caissons are grade beams, varying in depth from 2' to 4' depending on the location, which transfer façade and wall load to the foundation system. The ground (main) floor rests on a 6" concrete slab reinforced with W/4x4-W4.0x4.0 welded wire fabric.

The use of a deep foundation system with structural steel framing may seem odd at first glance, however site soils were reported to be very soft in nature. When the geotechnical investigation was done, it was found that within the first 12 feet below grade poorly graded sand with silt, silty sand, silty clay, and lean clays were found. Very stiff lean clays were encountered to a depth of 80 feet below existing grade. Below depths of 80 to 85 feet, very dense silty and sandy soils and heavily consolidated clayey soils were encountered in all borings to depths of 89 to 100 feet, where the drilling process was terminated. As a result, drilled caissons with belled bases were recommended for the foundation system based upon the soft clays found 12 to 40 feet below grade.

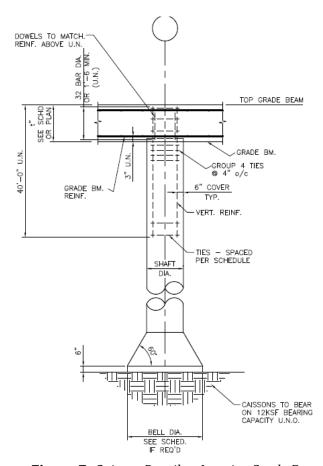


Figure 7: Caisson Detail at Interior Grade Beam

PROBLEM STATEMENT

Upon completion of studying the structural system with respect to gravity and lateral loads, it was determined that the existing structural system meets architectural, strength, and serviceability requirements for St. Vincent Mercy Medical Center Heart Pavilion. However, there is currently a non-seismic steel moment frame at every column line in both directions. This may seem odd and first glance, however, the nature of this site has a significant impact on the structural design of the building.

Based on field and laboratory test data within the geotechnical report for the site, it was determined that the soil located 12 to 40 feet below existing grade has an un-drained shear strength of less than 500 psf. For this reason, the soil is classified as very soft to soft lean clay and is characterized by the Ohio Building Code as Site Class E, "Soft Soil Profile". This significantly impacted the base shear value, leading to a seismically controlled building even when torsion effects were considered.

As stated in the geotechnical report, drilled caissons with belled bases were recommended for the foundation system based upon the soft clays found 12 to 40 feet below grade. Drilled shaft foundations obtain capacity from side resistance and end bearing. However, due to the nature of the soil within this site, capacity is obtained through bearing in the very stiff soils located 80 feet below grade.

Since this facility is ultimately controlled by seismic forces, seismic detailing of the steel moment frames would reduce the quantity of moment frames required to resist lateral forces. Even though seismically detailed connections are more expensive than typical connections, this could be a viable alternative for two reasons. First, the quantity of moment frames required would be greatly reduced. Second, if the moment frames were kept along the perimeter of the building, the size of interior members would be reduced. Ideally, this solution would optimize the lateral system while still meeting the demands of the seismically controlled site.

PROPOSED SOLUTION

Theme: Optimizing the Lateral System and Minimizing Interior Member Sizes

Structural Depth: New Lateral System Design

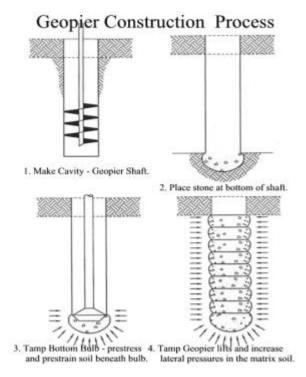
The current site of St. Vincent Mercy Medical Center Heart Pavilion was chosen by the owner because it was already owned by Mercy Health Partners and it is adjacent to the main hospital. As Toledo's first and only facility for the treatment of vascular disease, St. Vincent's Heart Pavilion has become a staple within the community. For these reasons, the Heart Pavilion will be kept on the existing site.

Due to the nature of the soil, steel is the best solution for a structural system. Concrete is a heavier material by nature and would only increase the weight of the building, creating an even higher base shear value. As a result, the Heart Pavilion will remain a composite steel structure.

Classified as Site Class E, the soil is very sensitive to seismic forces. As a result, it was necessary at the time of design to place non-seismic steel moment frames at every column in both directions. In order to optimize this lateral system, seismically detailed steel moment frames will be utilized to resist lateral loads. These seismically detailed steel moment frames will be placed along the perimeter of the building to ultimately reduce interior member sizes.

After designing the new lateral system, the seismically detailed connections will be designed based on applicable limit states. These connections will be detailed in AutoCAD and provided for reference with the new lateral system.

Within soft soil regions, geopiers are a great solution for a foundation system. They are installed by drilling a 30" diameter shaft and then ramming hard aggregate into the shaft in 12" lifts. This aggregate must be high quality and high hardness, or it will be destroyed. The ramming process is employed using a hydraulic hammer delivering 300 to 400 blows per minute. As seen in Figures 8 and 9 on the following page, this process increases the lateral pressures within the soil creating vertical reinforcement for the soil around the geopier. Installation of this system is very efficient as approximately 40 to 50 geo-piers can be installed per day.



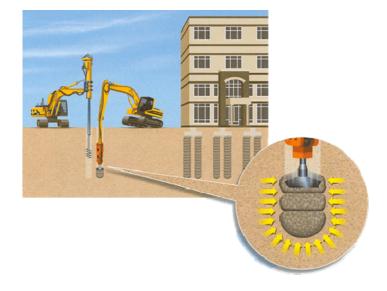


Figure 8: Detail of Ramming Process Photo courtesy of www.farrellinc.com

Figure 9: Geopier Installation Photo courtesy of www.farrellinc.com

A cost analysis will be prepared for the existing lateral system as well and the new lateral system in order to create a side by side comparison. Ideally, cost savings will be discovered with the new lateral system due to the reduced quantity of moment frames. However, availability of welders and special inspectors in the Toledo area must be considered within this cost analysis.

Breadth Study I: Façade Study

In the original design concepts, a surgery suit and conference spaces were located on the third floor. To provide natural day lighting to these spaces, a curtain wall is utilized along this entire level as seen in the picture to the right. However, the surgery spaces were later moved to a lower floor and patient rooms were added in its place. Since



the functionality of the third floor became just like the floors below it, the curtain wall system will be removed and the brick façade will continue up the entire height of the building. Once this change is implemented, heat loss calculations will be provided for the existing curtain wall system and for the new brick veneer system. Conclusions will then be drawn on which system is more efficient for the Heart Pavilion's needs.

Breadth Study II: Construction Management

Implementing special detailing of the lateral system will ultimately affect the construction schedule of the Heart Pavilion. Special welders would be required to install the seismically detailed connections. In addition, special inspectors must be available for regular visits during construction to ensure that the lateral system is being constructed properly. For this reason, a construction schedule will be provided for the existing lateral system as well as the proposed lateral system. Conclusions will then be drawn based on viability of the new lateral system with respect to cost and constructability.

TASKS & TOOLS

I. Structural Depth

A. Research Seismically Detailed Steel Moment Frames and Determine Feasibility

- 1. Task 1: Learn more about seismic detailing
 - a. Research similar buildings that utilize seismically detailed moment frames
 - b. Contact structural engineer to discuss advantages and disadvantages of this type of detailing
 - c. Determine if seismically detailed moment frames are still viable

B. Design Seismically Detailed Steel Moment Frames

- 1. Task 2: Build model in RAM to Design New Steel Structure
 - a. Set up existing column grids (since building geometry will be unchanged)
 - b. Create one file with non-seismic steel moment frames at every column line in both directions (existing lateral system)
 - c. Create a new file with seismically detailed steel moment frames along the perimeter of the building (proposed lateral system)

Milestone I: January 26: Making progress with structural modeling

- d. Ensure beams supporting I.C.U. and O.R. spaces are designed for vibration
- e. Input loads and compare both models
- 2. Task 3: Run RAM Frame (AE 597A)
 - a. Obtain periods of vibration of building
 - b. Note any torsion irregularities (if any) of proposed lateral system
 - c. Finalize new lateral system design

Milestone II: February 9: New lateral system designed

C. Design Seismically Detailed Connections for Steel Moment Frames

- 1. Task 4: Complete drawings of seismically detailed connections (AE 534)
 - a. Design seismically detailed connections based on limit states
 - b. Draw typical seismically detailed moment frame in AutoCAD and detail the designed connection

D. Design Geopier Foundation System

- 1. Task 5: Design geopiers
 - a. Contact GeoStructures to obtain information on the design of this system
 - b. Design geopier system based on professional input from GeoStructures
- 2. Task 6: Complete cost analysis of existing lateral/foundation system vs. new lateral/foundation system
 - a. Determine extra cost of seismically detailed steel moment frames with respect to special inspections and installation

b. Determine cost difference between drilled caissons and geopiers

Milestone III: February 23: Structural Depth Complete

II. Breadth Study I: Façade Study

- A. Task 1: Implement Façade Change
 - 1. Review details of the existing brick façade on the lower floors
 - 2. Model building to show brick façade extending up to the third floor
- B. Task 2: Prepare Heat Loss Comparison
 - 1. Calculate the temperature change across the existing curtain wall system
 - 2. Calculate the temperature change across the brick veneer system
 - 3. Draw conclusions on which system would provide more comfort to patients

Milestone IV: March 16: Breadth Study I Complete

III. Breadth Study II: Construction Management

- A. Task 1: Determine the Cost & Schedule of the Existing Lateral System
 - 1. Inquire to CM about any actual cost data or scheduling information that may be available
 - 2. Determine any cost data that was unavailable such as labor and material costs per RS Means
 - 3. Create a mock schedule of construction implementing the existing lateral system
- B. Task 2: Determine the Cost & Schedule of the Proposed Lateral System
 - 1. Determine extra cost for special welders to install seismically detailed connections and special inspectors to visit the construction site (may not be readily available in Toledo)
 - 2. Determine labor and material costs per RS Means
 - 3. Create a mock schedule of construction implementing the proposed lateral system
- C. Task 3: Comparison of Existing vs. Proposed Lateral Systems
 - 1. Compare cost information of the two lateral systems obtained in tasks 1 and 2 and determine and significant savings
 - 2. Compare schedules and total time of construction obtained in tasks 1 and 2 and determine and advantages or disadvantages of one system over the other
 - 3. Determine which system is more feasible with respect to constructability

TIMELINE

		Task	Jan 12-18	Jan 19-25	Jan 25- Feb 1	Feb 2-8	Feb 9-15	Feb 16-22	Feb 23- Mar 1	Mar 2-8
pth	A-1-a	Research Seismic Detailing								
	A-1-b	Determine Pros & Cons								
	A-1-c	Determine Viability								
	В-1-а	Set Up Column Grids in RAM								
	B-1-b	Create Model with Existing Lateral System								
	В-1-с	Create Model with Proposed Lateral System								
	B-1-d	Design Beams for Vibration in O.R. & I.C.U. Spaces								
	В-1-е	Input Loads & Compare Models								
ral De	B-2-a	Obtain Periods of Vibration								
Structural Depth	B-2-b	Note Any Torsion Irregularities								
	В-2-с	Finalize New Lateral System Design								
	C-1-a	Design Seismically Detailed Connections								
	C-1-b	Detail in AutoCAD								
	D-1-a	Contact GeoStructures								
	D-1-b	Design Geopiers								
	C-2-a	Determine Cost of Existing & Proposed Lateral/Foundation System								
	C-2-b	Compare Cost								
Breadth Study I	A-1	Review Existing Wall System Details								
	A-2	Model Building to show Façade Change								
	B-1	Determine Temp. Change across Existing Curtain Wall System								

	Task		Mar 9-15	Mar 16-22	Mar 23-29	Mar 30- Apr 5	Apr 6-12	April 13-19	Apr 20-26	Apr 27- May 3	May 4-11
Breadth Study I	B-2	Determine Temp. Change across Brick Veneer System	Spring Break					Final Presentations			
	В-3	Draw Conclusions on Occupant Comfort									
Breadth Study II	A-1	Obtain Actual Cost Data/ Scheduling Info from CM									
	A-2	Determine Labor/Mat'l Costs									
	A-3	Create Mock Schedule for Existing Lateral System									
	B-1	Determine Extra Costs for Proposed Lateral System									
	B-2	Determine Labor/Mat'l Costs									
	В-3	Create Mock Schedule of Proposed Lateral System									
	C-1	Compare Cost									
	C-2	Compare Construction Time									
	C-3	Determine System Feasibility									
Finalize Report and Presentation											
Post all Files to CPEP											
Complete ABET Evaluation											
Complete Final CPEP Update											

^{*}Weeks highlighted in orange indicate a milestone within that week.